

ET Epoxy-Tie® is a two-component, high solids, epoxy-based system for use as a high-strength, non-shrink anchor grouting material. Resin and hardener are dispensed and mixed simultaneously through the mixing nozzle. ET meets the ASTM C-881 specifications for Type I, II, IV and V, Grade 3, Classes B and C, except gel time.

- **USES:** Threaded rod anchoring
 - · Rebar doweling
 - · Bonding hardened concrete to hardened concrete
 - · Pick-proof sealant around doors, windows and fixtures
 - · Paste-over for crack injection

ICC-ES ER-4945 (URM); City of L.A. RR25185, RR25120; Florida FL 11506.2: Multiple DOT Listings. The load tables list values based upon results from the most recent testing and may not reflect those in current code reports. Where code jurisdictions apply, consult the current reports for applicable load values.

Surfaces to receive epoxy must be clean. For installations in or through standing water, see page 15 for details. The base material temperature must be 40°F or above at the time of installation. For best results, material should be 70°F - 80°F at the time of application. Cartridges should not be immersed in water to facilitate warming. To warm cold material, the cartridges should be stored in a warm, uniformly heated area or storage container for a sufficient time to allow epoxy to warm completely. Mixed material in nozzle can harden in 5-7 minutes at a temperature of 40°F or above.

NSTALLATION: See pages 31–32

24 months from date of manufacture in unopened container

AGE CONDITIONS: For best results store between 45°F - 90°F. To store partially used cartridges, leave hardened nozzle in place. To re-use, attach new nozzle.

COLOR: Resin – white, hardener – black

When properly mixed, ET adhesive will be a uniform medium gray color.

CLEAN UP: Uncured material — Wipe up with cotton cloths. If desired scrub area with abrasive, waterbased cleaner and flush with water. If approved, solvents such as ketones (MEK, acetone, etc.), lacguer thinner, or adhesive remover can be used. DO NOT USE SOLVENTS TO CLEAN ADHESIVE FROM SKIN. Take appropriate precautions when handling flammable solvents. Solvents may damage surfaces to which they are applied. Cured Material: Chip or grind off surface.

TEST CRITERIA: Anchors installed with ET Epoxy-Tie® adhesive have been tested in accordance with ICC-ES's Acceptance Criteria for Adhesive Anchors (AC58) for the following:

- · Seismic and wind Loading
- · Long-term creep at elevated-temperature
- · Static loading at elevated-temperature
- Damp holes
- · Freeze-thaw conditions
- · Critical and minimum edge distance and spacing

In addition, anchors installed with ET adhesive have been tested in accordance with ICC-ES's Acceptance Criteria for Unreinforced Masonry Anchors (AC60).

PROPERTY	TEST METHOD	RESULTS
Consistency (77°F)	ASTM C 881	Non-sag/thixotropic paste
Heat deflection	ASTM D 648	168°F (76°C)
Bond strength (moist cure)	ASTM C 882	2,030 psi (2 days) 4,240 psi (14 days)
Water absorption	ASTM D 570	0.19% (24 hours)
Compressive yield strength	ASTM D 695	9,174 psi (24 hours) 13,390 psi (7 days)
Compressive modulus	ASTM D 695	658,200 psi (7 days)
Gel time (77°F)	ASTM C 881	10 min. – 60 gram mass 30 min – Thin film

CAL RESISTANCE Very good to excellent against distilled water, inorganic acids and alkalis. Fair to good against organic acids and alkalis, and many organic solvents. Poor against ketones. For more detailed information visit our website or contact Simpson Strong-Tie and request Technical Bulletin T-SAS-CHEMRES08.



Model No.	Capacity ounces (cubic inches)	Cartridge Type	Carton Quantity	Dispensing Tool(s)	Mixing ⁴ Nozzle	
ET22	22 (39.7)	side-by-side	10	EDT22B, EDT22AP, or EDT22CKT	EMN22i	
ET56	56 (101.1)	side-by-side	6	EDT56AP	EMN22i or EMN50	

- 1. Bulk containers also available, call Simpson Strong-Tie for details.
- 2. Cartridge and bulk estimation guides are available on pages 63-66.
- 3. Detailed information on dispensing tools, mixing nozzles and other adhesive accessories is available on pages 87-92.
- 4. Use only appropriate Simpson Strong-Tie® mixing nozzle in accordance with Simpson Strong-Tie instructions. Modification or improper use of mixing nozzle may impair epoxy performance.

SUGGESTED SPECIFICATIONS: Anchoring adhesive shall be a twocomponent high solids epoxy based system supplied in manufacturer's standard side-by-side cartridge and dispensed through a static-mixing nozzle supplied by the manufacturer. Epoxy shall meet the minimum requirements of ASTM C-881 specification for Type I, II, IV, and V, Grade 3, Class B and C, except gel time, and must develop a minimum 13,390 psi compressive yield strength after 7 day cure. Epoxy must have a heat deflection temperature of a minimum 168°F (76°C). Adhesive shall be ET Epoxy-Tie® adhesive from Simpson Strong-Tie, Pleasanton, CA. Anchors shall be installed per Simpson Strong-Tie instructions for ET Epoxy-Tie® adhesive.

See pages 87–92 for information on dispensing tools. mixing nozzles and other accessories.



Cure Schedule

	Base Material Temperature					
°F	°C	Time				
40	4	72 hrs.				
60	16	24 hrs.				
80	27	24 hrs.				
100	38	12 hrs.				

In-Service Temperature Sensitivity

Base M Tempe		Percent Allowable
°F	°C	Load
40	4	100%
70	21	100%
110	43	100%
135	57	85%
150	66	69%
180	82	58%

- Refer to temperature-sensitivity chart for allowable bond strength reduction for temperature. See page 15 for more information.
- 2. Percent allowable load may be linearly interpolated for intermediate base material temperatures.
- 3. $^{\circ}$ C = ($^{\circ}$ F-32) / 1.8

Tension Loads for Threaded Rod Anchors in Normal-Weight Concrete







*See page 10 for an explanation of the load table icons

III INUI III	ai-wei	yılı Gülici	EIE						8M 83	
Rod Dia.	Drill Bit	Embed. Depth	Critical Edge	Critical Spacing		Tension Load I on Bond Str			Tension Load d on Steel Strer	ngth
in. (mm)	Dia. in.	in. (mm)	Dist.	Dist.		"c ≥ 2000 ps 8 MPa) Cond		A307 (SAE 1018)	A193 GR B7 (SAE 4140)	F593 (A304SS)
()		()	(mm)	(mm)	Ultimate lbs. (kN)	Std. Dev. lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)
3/8 (9.5)	1/2	3 ½ (89)	5 ½ (133)	14 (356)	8,777 (39.0)	324 (1.4)	2,195 (9.8)	2,105 (9.4)	4,535 (20.2)	3,630 (16.1)
1/2 (12.7)	5/8	4 ½ (108)	6 % (162)	17 (432)	15,368 (68.4)	605 (2.7)	3,840 (17.1)	3,750 (16.7)	8,080 (35.9)	6,470 (28.8)
5/8 (15.9)	3/4	5 (127)	7 ½ (191)	20 (508)	22,877 (101.8)	718 (3.2)	5,720 (25.4)	5,875 (26.1)	12,660 (56.3)	10,120 (45.0)
3/4 (19.1)	7/8	6 ¾ (171)	10 1/⁄8 (257)	27 (686)	35,459 (157.7)	4,940 (22.0)	8,865 (39.4)	8,460 (37.6)	18,230 (81.1)	12,400 (55.2)
7/8 (22.2)	1	7 ¾ (197)	11 % (295)	31 (787)	43,596 (193.9)	1,130 (5.0)	10,900 (48.5)	11,500 (51.2)	24,785 (110.2)	16,860 (75.0)
1 (25.4)	1 1// ₈	9 (229)	13 ½ (343)	36 (914)	47,333 (210.5)	1,243 (5.5)	11,835 (52.6)	15,025 (66.8)	32,380 (144.0)	22,020 (97.9)
1 ½ (28.6)	11/4	10 1/8 (257)	15 1⁄ ₄ (387)	40 ½ (1029)	61,840 (275.1)	•	15,460 (68.8)	19,025 (84.6)	41,000 (182.4)	27,880 (124.0)
1 ½ (31.8)	1%	11 ½ (286)	16 % (429)	45 (1143)	78,748 (350.3)	4,738 (21.1)	19,685 (87.6)	23,490 (104.5)	50,620 (225.2)	34,420 (153.1)
1 Allanuah	مم اممما مم		. af tha haad	or ctool etror						

- 1. Allowable load must the lesser of the bond or steel strength.
- 2. The allowable loads listed under allowable bond are based on a safety factor of 4.0.
- The anowable loads listed under anowable bond are based on a safety factor of 4.0.
 Allowable loads may be increased by 33½ percent for short-term loading due to wind or seismic forces where permitted by code.
 Refer to allowable load-adjustment factors for spacing and edge distance on pages 60 and 61.
 Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.

- Anchors are permitted to be used within fire-resistive construction, provided the anchors resist wind
 or seismic loads only. For use in fire-resistive construction, the anchors can also be permitted to be
 used to resist gravity loads, provided special consideration has been given to fire-exposure conditions.
 Anchors are not permitted to resist tension forces in overhead or wall installations unless proper
- consideration is given to fire-exposure and elevated-temperature conditions.

Strong-Tie

Shear Loads for Threaded Rod Anchors in Normal-Weight Concrete







*See page 10 for a
explanation of the
load table icons

Rod Dia.	Drill Bit	Embed. Depth	Critical Edge	Critical Spacing		ar Load Base ete Edge Dis		Shear Load Based on Steel Strength			
in. (mm)	Dia. in.	in. (mm)	Dist.	Dist.		f'c ≥ 2000 ps 8 MPa) Cond		A307 (SAE 1018)	A193 GR B7 (SAE 4140)	F593 (A304SS)	
,		, ,	(mm)	(mm)	Ultimate lbs. (kN)	Std. Dev. lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)	
3/8 (9.5)	1/2	3 ½ (89)	5 ½ (133)	5 ½ (133)	7,615 (33.9)	591 (2.6)	1,905 (8.5)	1,085 (4.8)	2,340 (10.4)	1,870 (8.3)	
1/2 (12.7)	5/8	4 ½ (108)	6 % (162)	6 % (162)	11,273 (50.1)	1,502 (6.7)	2,820 (12.5)	1,930 (8.6)	4,160 (18.5)	3,330 (14.8)	
5/8 (15.9)	3/4	5 (127)	7 ½ (191)	7 ½ (191)	19,559 (87.0)	1,289 (5.7)	4,890 (21.8)	3,025 (13.5)	6,520 (29.0)	5,220 (23.2)	
3/4 (19.1)	7/8	6 3⁄4 (171)	10 1/8 (257)	10 1/8 (257)	27,696 (123.2)	2,263 (10.1)	6,925 (30.8)	4,360 (19.4)	9,390 (41.8)	6,385 (28.4)	
7/8 (22.2)	1	7 3⁄4 (197)	11 % (295)	11 5/8 (295)	•	•	6,925 (30.8)	5,925 (26.4)	12,770 (56.8)	8,685 (38.6)	
1 (25.4)	11/8	9 (229)	13 ½ (343)	13 ½ (343)	53,960 (240.0)	3,821 (17.0)	13,490 (60.0)	7,740 (34.4)	16,680 (74.2)	11,345 (50.5)	
1 1/ ₈ (28.6)	11/4	10 1/8 (257)	15 1⁄ ₄ (387)	15 ½ (387)	59,280 (263.7)	•	14,820 (65.9)	9,800 (43.6)	21,125 (94.0)	14,365 (63.9)	
1 ½ (31.8)	1%	11 ½ (286)	16 7/8 (429)	16 % (429)	64,572 (287.2)	3,503 (15.6)	16,145 (71.8)	12,100 (53.8)	26,075 (116.0)	17,730 (78.9)	

- 1. Allowable load must be the lesser of the load based on concrete edge distance or steel strength.
- 2. The allowable loads based on concrete edge distance are based on a safety factor of 4.0.
- 3. Allowable loads may be increased by 331/3 percent for short-term loading due to wind or seismic forces where permitted by code.
- 4. Refer to allowable load-adjustment factors for spacing and edge distance on pages 60 and 61.
- 5. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.
- 6. Anchors are permitted to be used within fire-resistive construction, provided the anchors resist wind or seismic loads only. For use in fire-resistive construction, the anchors can also be permitted to be used to resist gravity loads, provided special consideration has been given to fire-exposure conditions.

Tension Loads for Threaded Rod Anchors in Normal-Weight Concrete Stemwall





*See page 10 for an

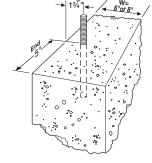
explanation of the

load table icons



Rod Dia	a. Drill Bit Dia.	Embed. Depth	Stemwall Width	III Min. Min. Bond Strength Steel Stre Edge End Dist. Dist. f'c ≥ 2000 psi A307		Bond Strength			Tension Load Based on Steel Strength
(mm)		in. (mm)	in. (mm)			A307 (SAE 1018)			
		(,	()	(mm)	(mm)	Ultimate lbs. (kN)	Std. Dev. Ibs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)
5/8 (15.9)	3/4	9 ½ (241.3)	6 (152.4)	1 ³ / ₄ (44.5)	5 (127.0)	10,720 (47.7)	1,559 (6.9)	2,680 (11.9)	5,875 (26.1)
5/8 (15.9)	3/4	12 (304.8)	6 (152.4)	1 3/ ₄ (44.5)	5 (127.0)	16,150 (71.8)	260 (1.2)	4,040 (18.0)	5,875 (26.1)
7/8 (22.2)	1	12 ½ (317.5)	8 (203.2)	1 3/ ₄ (44.5)	5 (127.0)	17,000 (75.6)	303 (1.3)	4,250 (18.9)	11,500 (51.2)
7/8 (22.2)	1	15½ (393.7)	8 (203.2)	1 ³ / ₄ (44.5)	5 (127.0)	23,340 (103.8)	762 (3.4)	5,835 (26.0)	11,500 (51.2)

- 1. Allowable load must be the lesser of the bond or steel strength.
- 2. The allowable loads listed under allowable bond are based on a safety factor of 4.0.
- 3. Allowable loads may be increased by 331/3 percent for short-term loading due to wind or seismic forces where permitted by code.
- 4. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.
- 5. Anchors are permitted to be used within fire-resistive construction, provided the anchors resist wind or seismic loads only. For use in fire-resistive construction, the anchors can also be permitted to be used to resist gravity loads, provided special consideration has been given to fire-exposure conditions.



Edge and end distances for threaded rod in concrete foundation stemwall corner installation



Tension Loads for Rebar Dowels in Normal-Weight Concrete







Rebar Size	Drill	Embed.	Critical	Critical		Tensi		Tension Load Based on Steel Strength			
No. (mm)	Bit Dia. in.	Depth in. (mm)	Edge Dist. in.	Spacing Dist. in.		f'c ≥ 2000 p .8 MPa) Con			f'c ≥ 4000 p .6 MPa) Con		ASTM A615 Grade 60 Rebar
(11111)		(111111)	(mm)	(mm)	Ultimate lbs. (kN)	Std. Dev. Ibs. (kN)	Allowable lbs. (kN)	Ultimate lbs. (kN)	Std. Dev. Ibs. (kN)	Allowable lbs. (kN)	Allowable lbs. (kN)
#4	5/8	4 ½ (108)	6 % (162)	17 (432)	17,596 (78.3)	533 (2.4)	4,400 (19.6)	•	•	4,400 (19.6)	4,800
(12.7)	78	6 (152)	9 (229)	24 (610)	•	•	•	20,250 (90.1)	263 (1.2)	5,060 (22.5)	(21.4)
#5	3/4	5 (127)	7 ½ (191)	20 (508)	25,427 (113.1)	1,899 (8.4)	6,355 (28.3)	•	•	6,355 (28.3)	7,440
(15.9)	94	9 % (238)	14 ½ (359)	37 ½ (953)	•	•	•	29,510 (131.3)	2,270 (10.1)	7,375 (32.8)	(33.1)
#6	7/8	6 ¾ (171)	10 1/⁄8 (257)	27 (686)	41,812 (186.0)	595 (2.6)	10,455 (46.5)	•	•	10,455 (46.5)	10,560
(19.1)	78	11 ½ (286)	16 % (429)	45 (1143)	•	•	•	44,210 (196.7)	1,227 (5.5)	11,050 (49.2)	(47.0)
#7	1	7 ¾ (197)	11 % (295)	31 (787)	50,241 (223.5)	2,995 (13.3)	12,560 (55.9)	•	•	12,560 (55.9)	14,400
(22.2)	'	13 1/⁄ ₈ (333)	19 ¾ (502)	52 ½ (1334)	•	•	•	59,325 (263.9)	3,444 (15.3)	14,830 (66.0)	(64.1)
		9 (229)	13 ½ (343)	36 (914)	60,145 (267.5)	5,493 (24.4)	15,035 (66.9)	•	•	15,035 (66.9)	
# 8 (25.4)	1 ½	12 (305)	18 (457)	48 (1219)	•	•	•	•	•	18,260 (81.2)	18,960 (84.3)
		15 (381)	22 ½ (572)	60 (1524)	•	•	•	85,970 (382.4)	17,965 (79.9)	21,490 (95.6)	
		9 (229)	13 ½ (343)	36 (914)	•	•	15,035 (66.9)	•	•	15,035 (66.9)	
# 9 (28.6)	11/4	13 (330)	19 ½ (495)	52 (1321)	•	•	•	•	•	21,310 (94.8)	24,000 (106.8)
		16 % (429)	25 % (645)	67 ½ (1715)	•	•	•	110,370 (491.0)	4,768 (21.2)	27,590 (122.7)	
		11 ½ (286)	16 % (429)	45 (1143)	70,685 (314.4)	1,112 (4.9)	17,670 (78.6)	•	•	17,670 (78.6)	
# 10 (31.8)	1½	15 (381)	22 ½ (572)	60 (1524)	•	•	•	•	•	23,960 (106.6)	30,480 (135.6)
		18 3/4 (476)	28 1/ ₈ (714)	75 (1905)	•	•	•	120,976 (538.1)	6,706 (29.8)	30,245 (134.5)	- (100.0)
		12 % (314)	18 5/8 (473)	49 ½ (1257)	78,422 (348.8)	4,603 (20.5)	19,605 (87.2)	•	•	19,605 (87.2)	
#11 (34.9)	1%	16 ½ (419)	24 ³ ⁄ ₄ (629)	66 (1676)	•	•	•	•	•	28,605 (127.2)	37,440 (166.5)
		20 5/8 (524)	31 (787)	82 ½ (2096)	•	•	•	150,415 (669.1)	8,287 (36.9)	37,605 (167.3)	
#14 (44.5)	2	15 3/4 (400)	23 5/8 (600)	63 (1600)	91,518 (407.1)	3,797 (16.9)	22,880 (101.8)	•	•	22,880 (101.8)	54,000 (240.2)

^{1.} Allowable load must be the lesser of the bond or steel strength.

*See page 10 for an explanation of the load table icons

^{2.} The allowable loads listed under allowable bond are based on a safety factor of 4.0.

^{3.} Allowable loads may be increased by 331/3 percent for short-term loading due to wind or seismic forces for only the shallowest embedment depths listed where permitted by code.

Refer to allowable load-adjustment factors for spacing and edge distance on pages 60 and 61.
 Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.

^{6.} Anchors are permitted to be used within fire-resistive construction, provided the anchors resist wind or seismic loads only. For use in fire-resistive construction, the anchors can also be permitted to be used to resist gravity loads, provided special consideration has been given to fire-exposure conditions.

^{7.} Anchors are not permitted to resist tension forces in overhead or wall installations unless proper consideration is given to fire-exposure and elevated-temperature conditions.

SIMPSON Strong-Tie ANCHOR SYSTEMS

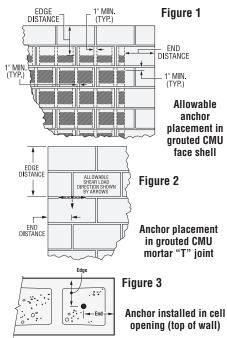
Shear Loads for Rebar Dowels in Normal-Weight Concrete

Rebar Size No.	Drill Bit Dia. in.	Embed. Depth in.	Critical Edge Dist.	Critical Spacing Dist.	Conci	ar Load Base rete Edge Dis f'c ≥ 2500 psi .2 MPa) Conc	tance	Shear Load Based on Steel Strength ASTM A615 Grade 60 Rebar
(mm)		(mm)	in. (mm)	in. (mm)	Ultimate lbs. (kN)	Std. Dev. lbs. (kN)	Allow. lbs. (kN)	Allowable lbs. (kN)
# 4 (12.7)	5/8	4 ½ (108)	8 (203)	6 % (162)	13,564 (60.3)	971 (4.3)	3,390 (15.1)	3,060 (13.6)
#5 (15.9)	3/4	5 (127)	10 (254)	7 ½ (191)	20,914 (93.0)	3,034 (13.5)	5,230 (23.3)	4,740 (21.1)
# 6 (19.1)	7/8	6 3⁄4 (171)	12 (305)	10 1/8 (257)	30,148 (134.1)	1,322 (5.9)	7,535 (33.5)	6,730 (29.9)
#7 (22.2)	1	7 3/4 (197)	14 (356)	11 % (295)	39,838 (177.2)	1,854 (8.2)	9,960 (44.3)	9,180 (40.8)
#8 (25.4)	11/8	9 (229)	16 (406)	13 ½ (343)	53,090 (236.2)	3,562 (15.8)	13,270 (59.0)	12,085 (53.8)
#9 (28.7)	11/4	10 1/ ₈ (257)	18 (457)	15 ½ (387)	63,818 (148.7)	3,671 (16.3)	15,955 (71.0)	15,300 (68.1)
# 10 (32.3)	1½	11 ½ (286)	20 (508)	16 % (429)	82,782 (368.2)	2,245 (10.0)	20,695 (92.1)	19,430 (86.4)
# 11 (35.8)	1%	12 % (314)	22 (559)	18 5/8 (473)	96,056 (427.3)	3,671 (16.3)	24,015 (106.8)	23,865 (106.2)
# 14 (43.0)	2	12 % (314)	22 (559)	18 % (473)	•	•	24,015 (106.8)	34,425 (153.1)

*See page 10 for an explanation of the load table icons

- Allowable load must be the lesser of the load based on concrete edge distance or steel strength.
- 2. The allowable loads based on concrete edge distance are based on a safety factor of 4.0.
- 3. Allowable loads must be decreased by 12 percent for short-term loading due to wind or seismic forces.
- 4. Refer to allowable load-adjustment factors for spacing and edge distance on pages 60 and 61.
- 5. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.
- 6. Anchors are permitted to be used within fire-resistive construction, provided the anchors resist wind or seismic loads only. For use in fire-resistive construction, the anchors can also be permitted to be used to resist gravity loads, provided special consideration has been given to fire-exposure conditions.

		ch Norma					nd 8-inch Gr	out Eillad C	MII
Rod Dia.	Drill Bit	Embed. Depth ⁸	Min. Edge	Min. End	Min. Spacing		le Loads Bas		
in.	Dia.	in.	Dist.	Dist.	Dist.	Ten	sion	Sh	ear
(mm)	in.	(mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate lbs. (kN)	Allowable lbs. (kN)	Ultimate lbs. (kN)	Allowable lbs. (kN)
			Ancho	r Installe	d in Face	Shell (See F	igure 1)		
3/8	7/	3½	12 (305)	4 (102)	14 (356)	6,489 (28.9)	1,300 (5.8)	5,231 (23.3)	1,045 (4.6)
(9.5)	7/16	(89)	12 (305)	12 (305)	14 (356)	7,247 (32.2)	1,450 (6.4)	6,738 (30.0)	1,350 (6.0)
1/2		41/4	12 (305)	4 (102)	17 (432)	8,646 (38.5)	1,730 (7.7)	5,705 (25.4)	1,140 (5.1)
(12.7)		(108)	12 (305)	12 (305)	17 (432)	8,975 (39.9)	1,795 (8.0)	10,879 (48.4)	2,175 (9.7)
		4 ³ ⁄ ₄ (121)	12 (305)	4 (102)	19 (483)	10,213 (45.4)	2,045 (9.1)	5,973 (26.6)	1,195 (5.3)
5/8 (15.9)	3/4		12 (305)	12 (305)	19 (483)	11,290 (50.2)	2,260 (10.1)	13,027 (57.9)	2,605 (11.6)
		6 (152)	4 (102)	4 5/8 (117)	24 (610)	4,905 (21.8)	980 (4.4)	4,766 (21.2)	955 (4.2)
3/4	7/	63/4	12 (305)	4 (102)	27 (686)	11,976 (53.3)	2,395 (10.7)	•	•
(19.1)	7/8	(171)	12 (305)	12 (305)	27 (686)	•	•	19,141 (85.1)	3,830 (17.0)
			Anchor I	nstalled i	n Mortar "	T" Joint (Se	e Figure 2)		
3/8 (9.5)	7/16	3 ½ (89)	8 (203)	8 (203)	14 (356)	7,646 (34.0)	1,530 (6.8)	5,507 (24.5)	1,100 (4.9)
½ (12.7)	9/16	4 ½ (108)	8 (203)	8 (203)	17 (432)	9,529 (42.4)	1,905 (8.5)	8,003 (35.6)	1,600 (7.1)
5/8 (15.9)	3/4	4 ³ ⁄ ₄ (121)	8 (203)	8 (203)	19 (483)	9,955 (44.3)	1,990 (8.9)	9,529 (42.4)	1,905 (8.5)
3/ ₄ (19.1)	7/8	6 ³ ⁄ ₄ (171)	16 (406)	8 (203)	27 (686)	•	•	7,238 (32.2)	1,450 (6.4)
		Anc		led in Cel	l Opening	(Top of Wall) (See Figur	e 3)	
5/8 (15.9)	3/4	6 (152)	4 (102)	4 5⁄ ₈ (117)	24 (610)	6,721 (29.9)	1,345 (6.0)	4,833 (21.5)	965 (4.3)
3/4 (19.1)	7/8	6 (152)	4 (102)	4 5⁄8 (117)	24 (610)	•	1,345 (6.0)	•	965 (4.3)



- Threaded rods must comply with ASTM A 307 minimum.
 Values for 6 and 8-inch wide CMU Grade N, Type Ilconcrete masonry units conforming to UBC Standard 21-4 or ASTM C90. The masonry units must be fully grouted with grout complying with UBC Section 2103.4, or IBC Section 2103.12. Mortar is prepared in accordance with Section 2103.3 of the UBC and UBC Standard 21-15, or IBC Section 2103.8. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.

 Sembedgered doubt in preserved from the auticie force of the processive strength of masonry.
- Embedment depth is measured from the outside face of the concrete masonry unit for installations through a face shell.
 Allowable loads may not be increased for short-
- term loading due to wind or seismic forces.

 5. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.
- 6. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC. For installations under the UBC use a safety factor of 4.0 (multiply the tabulated allowable loads by 1.25).

 7. Anchors must be spaced a minimum distance
- of four times the anchor embedment. 8. For embedment depths of 6¾", 8-inch-wide normalweight grout-filled CMU block must be used.



Tension and Shear Loads for Threaded Rod Anchors in Lightweight, Medium-Weight and Normal-Weight Hollow CMU



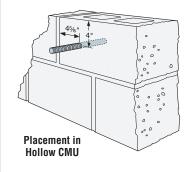
*See page 10 for explanation

of the load table icons

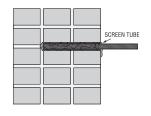
Rod Dia.	Drill Bit	Embed. Depth	Min. Edge Dist.	Min. End Dist.	6 and 8		v CMU Allowable Loads CMU Strength			
in.	Dia.	in.	in.	in.	Tension		Shear			
,	in.	(mm)	(mm)	(mm)	Ultimate lbs. (kN)	Allowable lbs. (kN)	Ultimate lbs. (kN)	Allowable lbs. (kN)		
		Anchor	Installed in Fa	ace Shell w/E	TS Screen T	ube (See Figu	re 1)			
1/2 (12.7)	5/8	3 (76.2)	4 (101.6)	4 5% (117.5)	1,400 (6.2)	280 (1.2)	1,326 (5.9)	265 (1.2)		
3/4 (19.1)	7/8	3 (76.2)	4 (101.6)	4 5% (117.5)	•	280 (1.2)	•	265 (1.2)		

- 1. Threaded rods must comply with ASTM A 307 minimum.
- 2. Values for 6 and 8-inch wide CMU Grade N, Type II concrete masonry units conforming to UBC Standard 21-4 or ASTM C90. Mortar is prepared in accordance with Section 2103.3 of the UBC and UBC Standard 21-15, or IBC Section 2103.8. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- 3. Embedment depth is measured from the outside face of the concrete masonry unit for installations through a face shell.
- 4. Allowable loads may not be increased for short-term loading due to wind or seismic forces.
- 5. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature
- 6. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC. For installations under the UBC use a safety factor of 4.0 (multiply the tabulated allowable loads by 1.25).
- 7. Anchors must be spaced a minimum distance of four times the anchor embedment.
- 8. Set drill to rotation-only mode when drilling into hollow CMU.

Figure 1



Configuration A (Shear)



Tension and Shear Loads for Installations in Unreinforced Brick Masonry Walls Minimum URM Wall Thickness is 13" (3 wythes thick)





of the load table icons

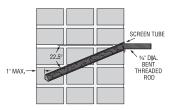




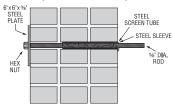
Rod Dia.	Drill Bit	Embed. Depth	Min. Edge/End	Min. Vertical	Min. Horiz.	Tension Load Based on URM Strength	Shear Load Based on URM Strength					
in. (mm)	Dia. in.	in. (mm)	Dist. in. (mm)	Spacing Dist. in.	Spacing Dist. in.	Minimum Net Mortar Strength = 50 psi	Minimum Net Mortar Strength = 50 psi					
			(,	(mm)	(mm)	Allowable lbs. (kN)	Allowable lbs. (kN)					
	Configuration A (Simpson ETS or ETSP Screen Tube Required)											
3/4 (19.1)	1	8 (203)	24 (610)	18 (457)	18 (457)	•	1,000 (4.4)					
		Con	figuration B	(Simpson	ETS or ETS	SP Screen Tube Required)					
3/4 (19.1)	1	13 (330)	16 (406)	18 (457)	24 (610)	1,200 (5.3)	1,000 (4.4)					
		Configurat	ion C (Simp	son ETS So	reen Tube	and AST Steel Sleeve Re	equired)					
5/8 (15.9)	1	**	24 (610)	18 (457)	18 (457)	1,200 (5.3)	750 (3.3)					
1. Threade	d rods m	ust comply	with ASTM A 3	307 minimun	n.	*Se	e page 10 for explanation					

- 1. Threaded rods must comply with ASTM A 307 minimum.
- 2. All holes are drilled with a 1" diameter carbide-tipped drill bit with the drill set in the rotation-only mode.
- 3. The unreinforced brick walls must have a minimum thickness of 13 inches (three wythes of brick).
- 4. The allowable load is applicable only where in-place shear tests indicate minimum net mortar strength of 50 psi.
- 5. The allowable load for Configuration B and C anchors subjected to a combined tension and shear load is determined by assuming a straight-line relationship between allowable tension and shear.
- 6. The anchors installed in unreinforced brick walls are limited to resisting seismic or wind forces only.
- 7. Configuration A has a straight threaded rod or rebar embedded 8 inches into the wall with a 31/32" diameter by 8-inch long screen tube (part # ETS758 or ETS758P). This configuration is designed to resist shear loads only.
- 8. Configuration B has a 34" threaded rod bent and installed at a 22.5-degree angle and installed 13 inches into the wall, to within 1-inch (maximum) of the exterior wall surface. This configuration is designed to resist tension and shear loads. The pre-bent threaded rod is installed with a 31/32" diameter by 13-inch long screen tube (part # ETS7513 or ETS7513P).
- 9. Configuration C is designed to resist tension and shear forces. It consists of a %" diameter, ASTM A 307 threaded rod and an 8" long sleeve (part # AST800) and a 31/32" diameter by 8-inch long screen tube (part # ETS758). The steel sleeve has a plastic plug in one end. A 6" by 6" by 3%" thick ASTM A 36 steel plate is located on the back face of the wall.
- 10. Special inspection requirements are determined by local jurisdiction and must be confirmed by the local building official.
- 11. Refer to in-service temperature sensitivity chart for allowable load adjustment for temperature.

Configuration B (Tension & Shear)



Configuration C (Tension & Shear)



Installation Instructions for Configuration C:

- 1. Drill hole perpendicular to the wall to a depth of 8" with a 1" diameter carbidetipped drill bit (rotation only mode).
- 2. Clean hole with oil-free compressed air and a nylon brush
- 3. Fill 8" steel screen tube with mixed adhesive and insert into hole.
- 4. Insert steel sleeve slowly into screen tube (adhesive will displace).
- 5. Allow adhesive to cure (see cure schedule).
- 6. Drill through plastic plug in (inside) end of steel sleeve with 5/8" bit.
- 7. Drill completely through the wall with 5/8" carbide tipped concrete drill bit (rotation mode only).
- 8. Insert %" rod through hole and attach metal plate and nut.



Load-Adjustment Factors for ET Epoxy-Tie® Adhesive in Normal-Weight Concrete: Edge Distance, Tension and Shear Loads

How to use these charts:

- 1. The following tables are for reduced edge distance.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- 4. Locate the edge distance (C_{act}) at which the anchor is to be installed. 5. The load-adjustment factor (f_c) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load-adjustment factor.
- Reduction factors for multiple edges are multiplied together.
- Adjustment factors do not apply to allowable steel strength values.
- Adjustment factors are to be applied to allowable Tension Load Based on Bond Strength values or allowable Shear Load Based on Concrete Edge Distance values only.

Edge Distance Tension (f_c)







*See page 10 for an explanation of the load table icons

	Dia.	3/8	1/2		5/8		3/4		7/8		1				11//8	11/4				
Edge	Rebar		#	4	#	5	#	6	#	7	#	8	#	9		#*	10	#1	11	#14
Dist.	E	31/2	41/4	6	5	9 3/8	63/4	111/4	73/4	131/8	9	15	9	16 7/8	101//8	111/4	18¾	12 %	20%	15¾
Cact	Ccr	51/4	63%	9	71/2	141/8	101/8	16 7/8	11%	193/4	131/2	221/2	131/2	25 %	151/4	16 7/8	28 1/8	18 %	31	23%
(in.)	C _{min}	13/4	13/4	13/4	13/4	13/4	13/4	13/4	13/4	13/4	13/4	13/4	2 3/4	2 3/4	2 3/4	2 3/4	23/4	23/4	23/4	23/4
	f _{cmin}	0.50	0.50	0.59	0.50	0.64	0.50	0.57	0.50	0.52	0.50	0.47	0.50	0.47	0.58	0.58	0.51	0.58	0.51	0.58
13/4		0.50	0.50	0.59	0.50	0.64	0.50	0.57	0.50	0.52	0.50	0.47								
23/4		0.64	0.61	0.65	0.59	0.67	0.56	0.60	0.55	0.55	0.54	0.50	0.50	0.47	0.58	0.58	0.51	0.58	0.51	0.58
3		0.68	0.64	0.66	0.61	0.68	0.57	0.61	0.56	0.55	0.55	0.50	0.51	0.48	0.59	0.59	0.51	0.59	0.51	0.59
4		0.82	0.74	0.72	0.70	0.71	0.63	0.63	0.61	0.58	0.60	0.53	0.56	0.50	0.62	0.62	0.53	0.61	0.53	0.61
5		0.96	0.85	0.77	0.78	0.73	0.69	0.66	0.66	0.61	0.64	0.55	0.60	0.52	0.66	0.65	0.55	0.64	0.55	0.63
6		1.00	0.96	0.83	0.87	0.76	0.75	0.69	0.72	0.63	0.68	0.58	0.65	0.55	0.69	0.68	0.57	0.67	0.57	0.65
7			1.00	0.89	0.96	0.79	0.81	0.72	0.77	0.66	0.72	0.60	0.70	0.57	0.72	0.71	0.59	0.69	0.58	0.67
8				0.94	1.00	0.82	0.87	0.75	0.82	0.69	0.77	0.63	0.74	0.59	0.76	0.74	0.61	0.72	0.60	0.69
9				1.00		0.85	0.93	0.78	0.87	0.71	0.81	0.66	0.79	0.62	0.79	0.77	0.63	0.75	0.62	0.71
10						0.88	0.99	0.80	0.92	0.74	0.85	0.68	0.84	0.64	0.82	0.80	0.65	0.77	0.64	0.73
12						0.94	1.00	0.86	1.00	0.79	0.94	0.73	0.93	0.69	0.89	0.86	0.69	0.82	0.67	0.77
14						1.00		0.92		0.85	1.00	0.78	1.00	0.73	0.96	0.91	0.73	0.88	0.71	0.81
16								0.98		0.90		0.83		0.78	1.00	0.97	0.77	0.93	0.74	0.85
18								1.00		0.95		0.89		0.83		1.00	0.80	0.98	0.77	0.89
20										1.00		0.94		0.87			0.84	1.00	0.81	0.93
22												0.99		0.92			0.88		0.84	0.97
24												1.00		0.97			0.92		0.88	1.00
26														1.00			0.96		0.91	
28																	1.00		0.95	
30																			0.98	
32																			1.00	

See notes below.

Edge Distance Shear (fc)

	→		*
4			

Dia.	3/8	1/2		5/8		3/4		7/8		1			1 1//8	11/4			
Rebar			#4		#5		#6		#7		#8	#9			#10	#11	#14
E	31/2	41/4	41/4	5	5	63/4	63/4	73/4	73/4	9	9	101/8	101//8	111/4	111/4	12 %	12 %
Ccr	51/4	63/8	8	71/2	10	101/8	12	11%	14	131/2	16	18	151/4	16 7/8	20	22	22
C _{min}	1 ¾	1 ¾	1 ¾	13/4	13/4	13/4	13/4	13/4	13/4	1 ¾	1 ¾	2 3/4	2 3/4	2 ¾	2 3/ ₄	2 ¾	23/4
f _{cmin}	0.29	0.20	0.16	0.13	0.10	0.13	0.10	0.13	0.09	0.08	0.08	0.08	0.14	0.14	0.14	0.14	0.14
	0.29	0.20	0.16	0.13	0.10	0.13	0.10	0.13	0.09	0.08	0.08						
	0.49	0.37	0.29	0.28	0.21	0.23	0.19	0.22	0.16	0.16	0.14	0.08	0.14	0.14	0.14	0.14	0.14
	0.54	0.42	0.33	0.32	0.24	0.26	0.21	0.24	0.18	0.18	0.16	0.10	0.16	0.16	0.15	0.15	0.15
	0.75	0.59	0.46	0.47	0.35	0.36	0.30	0.33	0.26	0.26	0.23	0.16	0.23	0.22	0.20	0.20	0.20
	0.95	0.76	0.60	0.62	0.45	0.47	0.39	0.42	0.33	0.33	0.29	0.22	0.29	0.28	0.25	0.24	0.24
	1.00	0.94	0.73	0.77	0.56	0.57	0.47	0.50	0.41	0.41	0.35	0.28	0.36	0.34	0.30	0.29	0.29
		1.00	0.87	0.92	0.67	0.68	0.56	0.59	0.48	0.49	0.42	0.34	0.43	0.40	0.35	0.33	0.33
			1.00	1.00	0.78	0.78	0.65	0.68	0.55	0.57	0.48	0.40	0.50	0.46	0.40	0.37	0.37
					0.89	0.88	0.74	0.77	0.63	0.65	0.55	0.46	0.57	0.52	0.45	0.42	0.42
					1.00	0.99	0.82	0.86	0.70	0.73	0.61	0.52	0.64	0.58	0.50	0.46	0.46
						1.00	0.91	0.94	0.78	0.80	0.68	0.58	0.71	0.64	0.55	0.51	0.51
							1.00	1.00	0.85	0.88	0.74	0.64	0.78	0.70	0.60	0.55	0.55
									0.93	0.96	0.81	0.70	0.85	0.76	0.65	0.60	0.60
									1.00	1.00	0.87	0.76	0.91	0.82	0.70	0.64	0.64
											0.94	0.82	0.98	0.89	0.75	0.69	0.69
											1.00	0.88	1.00	0.95	0.80	0.73	0.73
												1.00		1.00	0.90	0.82	0.82
															1.00	0.91	0.91
																1.00	1.00
	Rebar E C _{cr}	Rebar E 3½ C _{cr} 5¼ C _{min} 1¾ f _{cmin} 0.29 0.49 0.54 0.75 0.95	Rebar 8 E 3½ 4¼ C _{cr} 5¼ 6% C _{min} 1¾ 1¾ f _{emin} 0.29 0.20 0.29 0.20 0.37 0.54 0.42 0.75 0.95 0.76 1.00 1.00 0.94	Rebar #4 E 3½ 4¼ 4¼ C _{cr} 5¼ 6% 8 C _{min} 1¾ 1¾ 1¾ f _{emin} 0.29 0.20 0.16 0.29 0.20 0.16 0.49 0.37 0.29 0.54 0.42 0.33 0.75 0.59 0.46 0.95 0.76 0.60 1.00 0.94 0.73 1.00 0.87	Rebar #4 E 3½ 4¼ 4¼ 5 C _{cr} 5¼ 6% 8 7½ C _{min} 1¾ 1¾ 1¾ 1¾ f _{emin} 0.29 0.20 0.16 0.13 0.29 0.29 0.20 0.16 0.13 0.49 0.37 0.29 0.28 0.54 0.42 0.33 0.32 0.75 0.59 0.46 0.47 0.95 0.76 0.60 0.62 1.00 0.94 0.73 0.77 1.00 0.87 0.92	Rebar #4 #5 E 3½ 4¼ 4¼ 5 5 C _{er} 5¼ 6% 8 7½ 10 C _{min} 1¾ 1¾ 1¾ 1¾ 1¾ f _{emin} 0.29 0.20 0.16 0.13 0.10 0.29 0.20 0.16 0.13 0.10 0.49 0.37 0.29 0.28 0.21 0.54 0.42 0.33 0.32 0.24 0.75 0.59 0.46 0.47 0.35 0.95 0.76 0.60 0.62 0.45 1.00 0.94 0.73 0.77 0.56 1.00 0.87 0.92 0.67 1.00 0.87 0.92 0.67 1.00 0.89 0.89	Rebar #4 #5 E 3½ 4¼ 4¼ 5 5 6¾ C _{cr} 5¼ 6¾ 8 7½ 10 10½ C _{min} 1¾ 1 1 1 1 1 1 1 1	Rebar #4 #5 #6 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ C _{cr} 5¼ 6¾ 8 7½ 10 10½ 12 C _{min} 1¾ 1½ 1½ 1	Rebar #4 #5 #6 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ C _{cr} 5¼ 6% 8 7½ 10 10% 12 11% C _{min} 1¾ 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ C_{cr} 5¼ 6% 8 7½ 10 10% 12 11% 14 C_{min} 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11% 14 13½ C_{min} 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 #8 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 C_{min} 1¾ <td< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 18 C_{min} 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ <t< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C_{min} 1¾</td><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C_{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼</td></t<></td></td<></td></t<></td></t<></td></t<>	Rebar #4 #5 #6 #7 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ C _{cr} 5¼ 6% 8 7½ 10 10% 12 11% 14 C _{min} 1¾ 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11% 14 13½ C_{min} 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 #8 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 C_{min} 1¾ <td< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 18 C_{min} 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ <t< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C_{min} 1¾</td><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C_{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼</td></t<></td></td<></td></t<></td></t<>	Rebar #4 #5 #6 #7 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 C _{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11% 14 13½ C _{min} 1¾ 1¾ <t< td=""><td>Rebar #4 #5 #6 #7 #8 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 C_{min} 1¾ <td< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 18 C_{min} 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ <t< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C_{min} 1¾</td><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C_{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼</td></t<></td></td<></td></t<>	Rebar #4 #5 #6 #7 #8 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 C _{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 C _{min} 1¾ 1¾ <td< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ C_{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 18 C_{min} 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ <t< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C_{min} 1¾</td><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C_{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼</td></t<></td></td<>	Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ C _{cr} 5¼ 6% 8 7½ 10 10½ 12 11½ 14 13½ 16 18 C _{min} 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ <t< td=""><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C_{min} 1¾</td><td>Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C_{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C_{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼</td><td>Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼</td></t<>	Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7½ 7¾ 9 9 10½ 10½ C _{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ C _{min} 1¾ 1¾	Rebar #4 #5 #6 #7 #8 #9 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¼ 7¾ 9 9 10½ 10½ 11¼ C _{cr} 5¼ 6¾ 8 7½ 10 10½ 12 11½ 14 13½ 16 18 15¼ 16% C _{min} 1¾ 1¼ 1¾ 1¾ 1¾ 1¾ 1¾ 1¾ 1¼ 1¼ 1¼ 1¼	Rebar #4 #5 #6 #7 #8 #9 #10 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 11¼	Rebar #4 #5 #6 #7 #8 #9 #10 #11 E 3½ 4¼ 4¼ 5 5 6¾ 6¾ 7¾ 7¾ 9 9 10½ 10½ 11¼

- 1. E = Embedment depth (inches).
- 2. C_{act} = actual edge distance at which anchor is installed (inches). 3. C_{cr} = critical edge distance for 100% load (inches).
- 4. C_{min} = minimum edge distance for reduced load (inches).
- 5. f_c = adjustment factor for allowable load at actual edge distance.
- 6. f_{ccr} = adjustment factor for allowable load at critical edge distance. f_{ccr} is always = 1.00.
- 7. f_{cmin} = adjustment factor for allowable load at minimum edge distance.
- 8. $f_c = f_{cmin} + [(1 f_{cmin}) (C_{act} C_{min}) / (C_{cr} C_{min})].$

SIMPSON StrongTie ANCHOR SYSTEMS

explanation of the load table icons

Load-Adjustment Factors for ET Epoxy-Tie® Adhesive in Normal-Weight Concrete: Spacing, Tension and Shear Loads

How to use these charts:

- 1. The following tables are for reduced spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- 4. Locate the spacing (S_{act}) at which the anchor is to be installed.
- 5. The load-adjustment factor (f_s) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load-adjustment factor.
- 7. Reduction factors for multiple spacings are multiplied together.
- 8. Adjustment factors do not apply to allowable steel strength values.
- Adjustment factors are to be applied to allowable Tension Load Based on Bond Strength values or allowable Shear Load Based on Concrete Edge Distance values only.

Spacing Tension (fs)



																			_	
	Dia.	3/8	1/2		5/8		3/4		7/8		1				11/8	11/4				
	Rebar		#	4	#	5	#	6	#	7	#	8	#	9		#*	10	#1	11	#14
Sact	E	31/2	41/4	6	5	93%	6¾	111/4	73/4	131/8	9	15	9	167/8	101//8	111/4	18¾	12 %	20%	15¾
(in.)	Scr	14	17	24	20	371/2	27	45	31	521/2	36	60	36	671/2	401/2	45	75	491/2	821/2	63
	Smin	13/4	21/8	3	21/2	43/4	3%	5 %	37/8	6%	41/2	71/2	41/2	81/2	51/8	5 5// ₈	9%	61/4	10%	7 7/8
	f _{smin}	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
13/4		0.89																		
2		0.89																		
4		0.91	0.90	0.90	0.90		0.89		0.89											
6		0.93	0.92	0.91	0.91	0.89	0.90	0.89	0.90		0.90		0.90		0.89	0.89				
8		0.95	0.93	0.92	0.92	0.90	0.91	0.90	0.91	0.89	0.90	0.89	0.90		0.90	0.90		0.89		0.89
10		0.96	0.95	0.93	0.94	0.91	0.92	0.90	0.91	0.90	0.91	0.90	0.91	0.89	0.91	0.90	0.89	0.90		0.89
12		0.98	0.96	0.94	0.95	0.91	0.93	0.91	0.92	0.90	0.92	0.90	0.92	0.90	0.91	0.91	0.89	0.90	0.89	0.90
14		1.00	0.98	0.95	0.96	0.92	0.94	0.91	0.93	0.91	0.92	0.90	0.92	0.90	0.92	0.91	0.90	0.91	0.90	0.90
16			0.99	0.96	0.97	0.93	0.95	0.92	0.94	0.91	0.93	0.91	0.93	0.90	0.92	0.92	0.90	0.91	0.90	0.91
18			1.00	0.97	0.99	0.93	0.96	0.92	0.95	0.92	0.94	0.91	0.94	0.91	0.93	0.92	0.90	0.92	0.90	0.91
20				0.98	1.00	0.94	0.97	0.93	0.96	0.92	0.94	0.92	0.94	0.91	0.94	0.93	0.91	0.92	0.90	0.91
24				1.00		0.95	0.99	0.94	0.97	0.93	0.96	0.92	0.96	0.92	0.95	0.94	0.91	0.94	0.91	0.92
28						0.97	1.00	0.95	0.99	0.94	0.97	0.93	0.97	0.93	0.96	0.95	0.92	0.95	0.92	0.93
32						0.98		0.96	1.00	0.95	0.99	0.94	0.99	0.93	0.97	0.96	0.93	0.96	0.92	0.94
36						0.99		0.97		0.96	1.00	0.95	1.00	0.94	0.99	0.97	0.93	0.97	0.93	0.95
40						1.00		0.99		0.97		0.96		0.95	1.00	0.99	0.94	0.98	0.94	0.95
45								1.00		0.98		0.97		0.96		1.00	0.95	0.99	0.94	0.96
50										0.99		0.98		0.97			0.96	1.00	0.95	0.97
55										1.00		0.99		0.98			0.97		0.96	0.98
60												1.00		0.99			0.97		0.97	0.99
65														1.00			0.98		0.97	1.00
70																	0.99		0.98	
75																	1.00		0.99	
821/2																			1.00	
0 11	too Dolou																		•	

See Notes Below

Spacing Shear (f_s)

7		IU UI
2000		Ĩ←→Ĭ
2007 8003	37.50	15300000000

	Dia.	3/8	1/2	5/8	3/4	7/8	1	1 1//8	11/4		
	Rebar		#4	#5	#6	#7	#8	#9	#10	#11	#14
Sact	E	31/2	41/4	5	63/4	73/4	9	101/8	111/4	12 %	12 %
(in.)	Scr	51/4	6 3/8	71/2	101//8	11%	131/2	151/4	16%	18 5//8	18 5//8
	Smin	13/4	2 1/8	21/2	3 3/8	3 7/8	41/2	5 1/⁄8	5 5/8	61/4	61/4
	f _{smin}	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
13/4		0.83									
2		0.84									
3		0.89	0.87	0.85							
4		0.94	0.91	0.88	0.85	0.83					
5		0.99	0.95	0.92	0.87	0.85	0.84				
6		1.00	0.99	0.95	0.90	0.88	0.86	0.84	0.84		
7			1.00	0.98	0.92	0.90	0.88	0.86	0.85	0.84	0.84
8				1.00	0.95	0.92	0.90	0.88	0.87	0.85	0.85
9					0.97	0.94	0.92	0.90	0.88	0.87	0.87
10					1.00	0.96	0.93	0.91	0.90	0.88	0.88
12						1.00	0.97	0.95	0.93	0.91	0.91
14							1.00	0.98	0.96	0.94	0.94
16								1.00	0.99	0.96	0.96
18									1.00	0.99	0.99
20										1.00	1.00

*See page 10 for an explanation of the load table icons

- 1. E = Embedment depth (inches).
- 2. S_{act} = actual spacing distance at which anchors are installed (inches).
- 3. S_{cr} = critical spacing distance for 100% load (inches).
- 4. S_{min} = minimum spacing distance for reduced load (inches).
- 5. f_s = adjustment factor for allowable load at actual spacing distance.
 6. f_{scr} = adjustment factor for allowable load at
- critical spacing distance. f_{scr} is always = 1.00. 7. f_{smin} = adjustment factor for allowable load at minimum spacing distance.
- 8. $f_s = f_{smin} + [(1 f_{smin}) (S_{act} S_{min}) / (S_{cr} S_{min})].$